

## HPSBML Helical Slab Bracket with HP287/288 Helical Pier Systems Technical Specifications

<b>Bracket:</b>	Weldment manufactured from MC4 x 13.8 x 14.50" long steel channel and (2) - 0.50" thick x 2.50" x 6.00" steel plates. ASTM A36, yield strength = 36 ksi (min.).
<b>Guide Sleeve:</b>	Ø3.50" x 0.216" wall x 9.0" long. ASTM A500 Grade B or C, yield strength = 50 ksi (min.). 1.0" thick x 4.0" square tapped steel plate. ASTM A36, yield strength = 36 ksi (min.).
<b>Bracket Hardware:</b>	Ø1-1/4" x 6.00" long Grade 5 fully threaded bolt. Ø2.90" x 0.63" thick mechanical lift insert disc. Yield strength = 60 ksi (min.).
<b>Finish:</b>	Available plain or with hot-dip galvanized coating in accordance with ASTM A123 and ASTM A153.
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<b>Shaft Material:</b>	Ø2.875" x 0.203" wall or Ø2.875" x 0.276" wall ASTM A500 Grade B or C, yield strength = 60 ksi (min.).
<b>Helix Plates:</b>	ASTM A572 Grade 50 x 0.375" thick (standard). Helix plate geometry in accordance with ICC-ES AC358. Size, quantity and spacing of helix plates varies.
<b>Coupler Material:</b>	Ø3.50" x 0.281" wall x 6" long. ASTM A513 Type 5 Grade 1026, yield strength = 70 ksi (min.).
<b>Coupling Hardware:</b>	(2) - Ø3/4" zinc plated Grade 8 bolts with nuts.
<b>Finish:</b>	Available plain or with hot-dip galvanized coating in accordance with ASTM A123.



### Allowable System Capacity: 12,000 lbs.

**Notes:**

1. Capacity listed is a mechanical system capacity only. Local punching shear and slab strength should be checked separately. Reference FSI's slab pier spacing table for general spacing guidelines. Underside of slabs should be void filled after any piercing operation.
2. Capacity of the system may be governed by the capacity of the helical pier determined by torque correlation, field testing, or calculation by approved methods. See the FSI Technical Manual current edition for more information.
3. Mechanical capacity is based on continuous lateral soil confinement in soils with SPT blow counts  $\geq 4$ . Piles with exposed unbraced lengths or piles placed in weaker or fluid soils should be evaluated on a case by case basis by the project engineer.

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